

TRANSPORTATION IMPACT STUDY

FOR

LOFTS AT KALAMA

OLD PACIFIC HIGHWAY S NORTH OF ROBB ROAD

CITY OF KALAMA, WASHINGTON



6/15/2021

PREPARED BY

KELLY ENGINEERING

June 2021

TRANSPORTATION IMPACT STUDY

Lofts at Kalama

City of Kalama, Washington

June 15, 2021

Prepared for:

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TRANSPORTATION IMPACT STUDY

Lofts at Kalama

June 15, 2021

INTRODUCTION

A transportation impact study (TIS) for a 311 unit apartment complex was conducted to determine the potential traffic related impacts of the development to the surrounding roadway system. The development will consist of three and four story buildings. The TIS for the Lofts at Kalama was based on correspondence with representatives from Gray & Osborne, Inc. Gray & Osborne, Inc. are transportation engineering consultants for the City of Kalama.

The site is located on the west side of Old Pacific Highway S and approximately ¼ mile south of Cloverdale Road in the City of Kalama. The zoning designation is R-3 (high density residential). The tax lot is 411460100. The property is undeveloped with no existing buildings or structures. The vegetation is grass and scattered trees.

Land uses in the vicinity of the site consist of single-family homes and vacant land. The future Cedar Springs Subdivision will be constructed on the opposite side of Old Pacific Highway S. A vicinity map, aerial photograph and pre-app submittal site plans are shown in Figures 1a, 1b, 1c(a), 1c(b) and 1c(c).

Roadway Characteristics

The site will have four accesses onto Old Pacific Highway S. Old Pacific Highway S is a two lane paved arterial roadway with intermittent gravel/grass shoulders. The posted speed limit along the site frontage is 35 mph.

The Elm Street/1st Street, Robb Road/Old Pacific Highway S, I-5 NB and SB on/off ramps at Robb Road and the site access(s) onto Old Pacific Highway S were analyzed in this report. The Elm Street/1st Street intersection is controlled by stop signs on all approaches. The Robb Road/Old Pacific Highway S intersection is controlled by stop signs on the northbound, southbound and westbound approaches. The I-5 NB and SB ramps at Robb Road are controlled by stop signs at the off ramps. The lane configurations for the intersections are shown in Figure 2.

Traffic Volumes

The traffic counts in this report were conducted from 7:00 to 9:00 am and 4:00 to 6:00 pm during March 2018 and June 2021. The AM peak hour occurred between approximately 7:30 to 8:30 am and the PM peak hour occurred between approximately 4:40 to 5:40 pm. The peak hour at the intersections is the one-hour time period when traffic volumes are the highest and congestion is most likely to occur. The existing traffic volumes are shown in Figure 3. The raw traffic count data is shown in Appendix A.

As a result of the current service oriented facility and other closures required by Washington State due to the Covid 19 virus there has been a noticeable decline in traffic volumes. Therefore, traffic counts obtained during 2018 were increased by a factor of 6% to current year. The adjustments were based on discussions with representatives from Gray & Osborn Inc. Traffic counts obtained during 2021 at the I-5 northbound and southbound on/off ramps were adjusted as based on the historical traffic counts. The adjusted traffic volumes are shown in Figure 4.

Trip Generation/Distribution

The Lofts at Kalama will generate approximately 1,692 trips per day, ITE Trip Generation Manual, 10th edition. A trip is a one-directional vehicle movement. 112 trips will occur during the AM peak hour and 137 trips will occur during the PM peak hour. The trip generation rates are shown in Table 1.

Table 1
Site Traffic Generation

Land Use	ITE code	Trip Generation	Units	Trips/Day	Trips/AM Peak	Trips/PM Peak
Multifamily Housing (Mid-Rise)	221	5.44 trips/d.u.-Day 0.36 trips/d.u.-AM peak hour 0.44 trips/d.u.-PM peak hour	311	1,692	112 (in-28, out-84)	137 (in-83, out-54)

The directional distribution of traffic generated by the development was assigned to the study area intersections and site access(s). The distribution was based on the existing traffic counts and a survey conducted within the vicinity of the site. The site traffic distribution and assignment diagrams are shown in Figures 7a and 7b.

Year 2026 Traffic Volumes

The year 2026 traffic volumes at the study area intersections included a 2.0 percent per year compounded growth rate over the adjusted existing traffic volumes. The growth rate was based on discussions with representatives from Gray & Osborne, Inc. The year 2026 traffic volumes also included traffic from the future Cedar Springs Subdivision. The growth rate and in-process traffic from the Cedar Springs Subdivision was included to provide an analysis of the intersections for build-out of the Lofts at Kalama, forecast year 2026 traffic conditions.

Peak Hour Traffic Operations

The scope of the transportation impact study was based on discussions with representatives from Gray & Osborne, Inc. Based on the discussions the following intersections were analyzed in this report:

- (1) 1st Street & Elm Street.
- (2) Old Pacific Highway S & Robb Road
- (3) I-5 NB ramps at Robb Road
- (4) I-5 SB ramps at Robb Road
- (5) Old Pacific Highway S & site access(s) (future)

The intersections were analyzed to determine existing, year 2026 without project and year 2026 with project conditions. The site access(s) onto Old Pacific Highway S were analyzed for the year 2026 with and without project conditions. The assumption was made that site build out will occur within a five year time period. The year 2026 traffic volumes without the project are shown in Figure 6. The year 2026 traffic volumes with the project are shown in Figure 8.

The HCS + software was used to determine the level of service at the study area intersections. The software program is based on the Highway Capacity Manual methodology.

The Highway Capacity Manual procedures describe the operation of an intersection in terms of its level of service (LOS). The LOS criteria ranges from "A", which indicates little, if any, delay to "F", which indicates that vehicles experience very long delays. The LOS criteria with the corresponding delay in seconds per vehicle and capacity analysis summary are shown in Tables 2 and 3 on page 4.

Table 2
Level of Service Criteria

Level of Service (LOS)	A	B	C	D	E	F
Unsignalized Intersections Average Delay (seconds per vehicle)	≤10	>10-15	>15-25	>25-35	>35-50	>50

Table 3
Capacity Analysis Summary

	AM Peak Hour		PM Peak Hour	
	LOS	Delay	LOS	Delay
<i>1st Street & Elm Street</i>				
Existing	B	12.3	B	10.4
Year 2026 w/o project	B	14.4	B	11.5
Year 2026 with project	C	15.7	B	12.4
<i>Robb Road & Old Pacific Highway S</i>				
Existing	A	8.4	A	9.1
Year 2026 w/o project	A	8.6	A	9.7
Year 2026 with project	A	8.8	B	10.9
<i>I-5 NB ramps & Robb Road</i>				
Existing	A	9.7	B	10.6
Year 2026 w/o project	A	9.9	B	11.1
Year 2026 with project	B	10.1	B	11.7
<i>I-5 SB ramps & Robb Road</i>				
Existing	B	12.2	B	12.1
Year 2026 w/o project	B	13.2	B	13.1
Year 2026 with project	B	14.8	B	14.6
<i>Old Pacific Highway S & site access(s)</i>				
Year 2026 w/o Project	A	9.0	A	9.0
Year 2026 with project	A	9.4	A	9.8
LOS = Level of Service				
Delay = Approach Delay in seconds per vehicle				

Based on the findings of this transportation impact study the study area intersections will operate at LOS “C” or better with build-out of the Lofts at Kalama during the AM and PM peak hours. The LOS computer printouts are included in Appendix D.

Turn Lanes

The requirement for turn lanes was not conducted at the site access(s) due to low traffic volumes on S Pacific Highway and a very acceptable projected level of service with build out of the development.

Sight Distance

Sight distance was measured at the location of the future access(s) onto Old Pacific Highway S. The measured corner sight distance was over 400 feet when looking towards the north and south. A distance of 390 feet is required as based on the posted speed limit of 35 mph on Old Pacific Highway S and the criteria in AASHTO, A Policy on Geometric Design of Highways and Streets, 2010. Therefore, the sight distance requirement is met.

Collision Records

Collision records were obtained from the Washington State Department of Transportation (WSDOT) for the three year time period between June 1, 2018 and June 1, 2021. Based on the findings the calculated accident rates are below the threshold of 1.0 accidents per million entering vehicles (MEV) that usually identifies an intersection with a high accident rate. The collision data is shown in Table 4 and Appendix C.

Table 4
Collision Data

Intersection	Number of Collisions	Collision Type			Rate *
		Angle	Opposite Direction	Other	
Elm St./1 st St.	3	3		0	0.40
Old Pacific Hwy S/Robb Rd.	0				
I-5 NB ramps & Robb Rd.	2	1	1	0	0.34
I-5 SB ramps & Robb Rd.	0				

* Accident rate per million entering vehicles

Pedestrian/Bicycle/Transit Considerations

No pedestrian or bicycle activities were observed within the vicinity of the site. Sidewalks and bike lanes will be provided for along the site frontage of Old Pacific Highway S. The site is not served by public transit service.

CONCLUSIONS AND RECOMMENDATIONS

Based on the findings of this transportation impact study the surrounding roadway system can adequately accommodate traffic from the Lofts at Kalama. The study area intersections will operate at LOS "C" or better with build-out of the development.

Adequate sight distance should be maintained at the site access onto Old Pacific Highway S. Obstructions by landscaping, signs, parked vehicles or other objects should not be allowed.

No additional off-site traffic control devices were identified to accommodate the development.

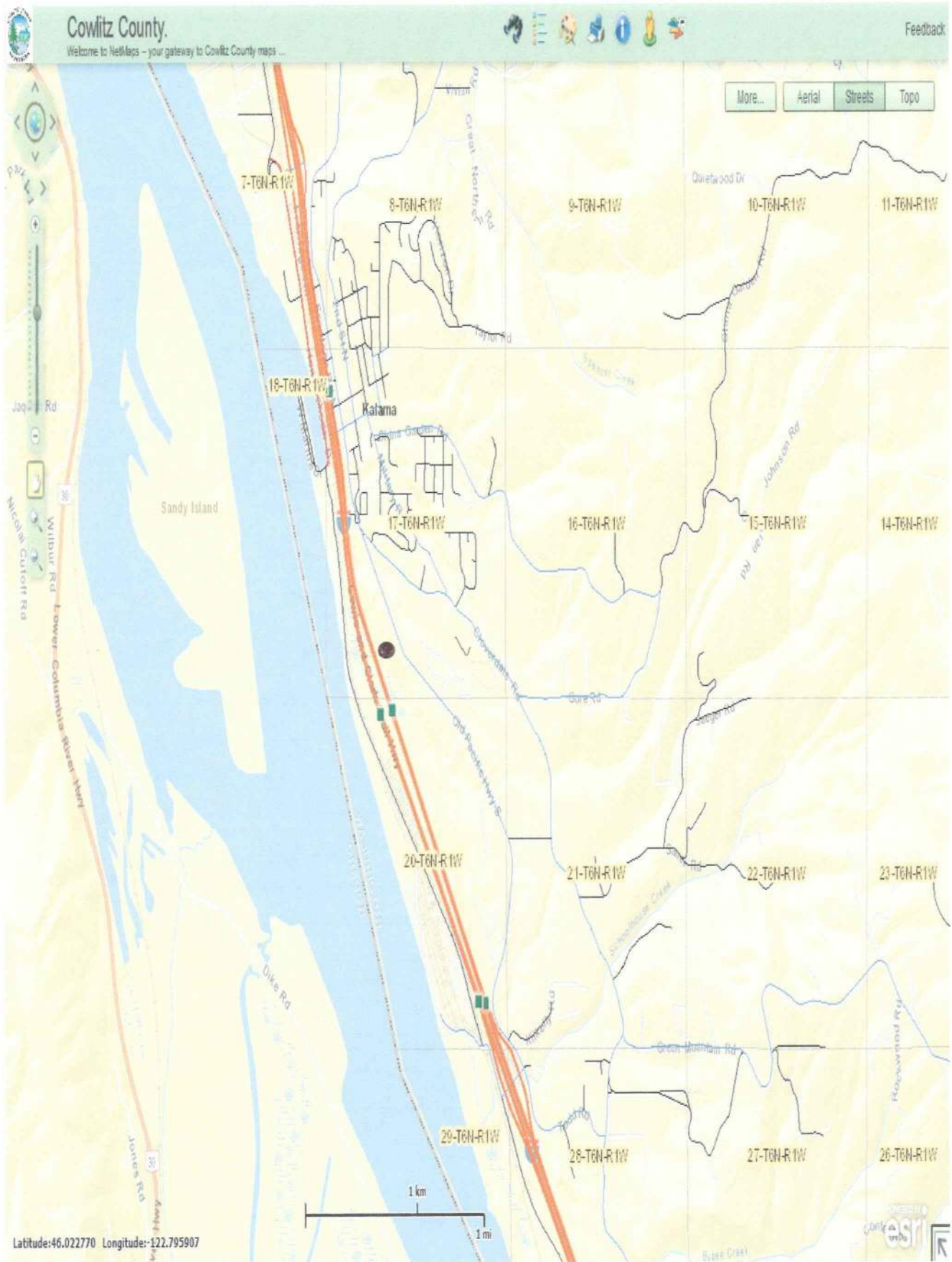
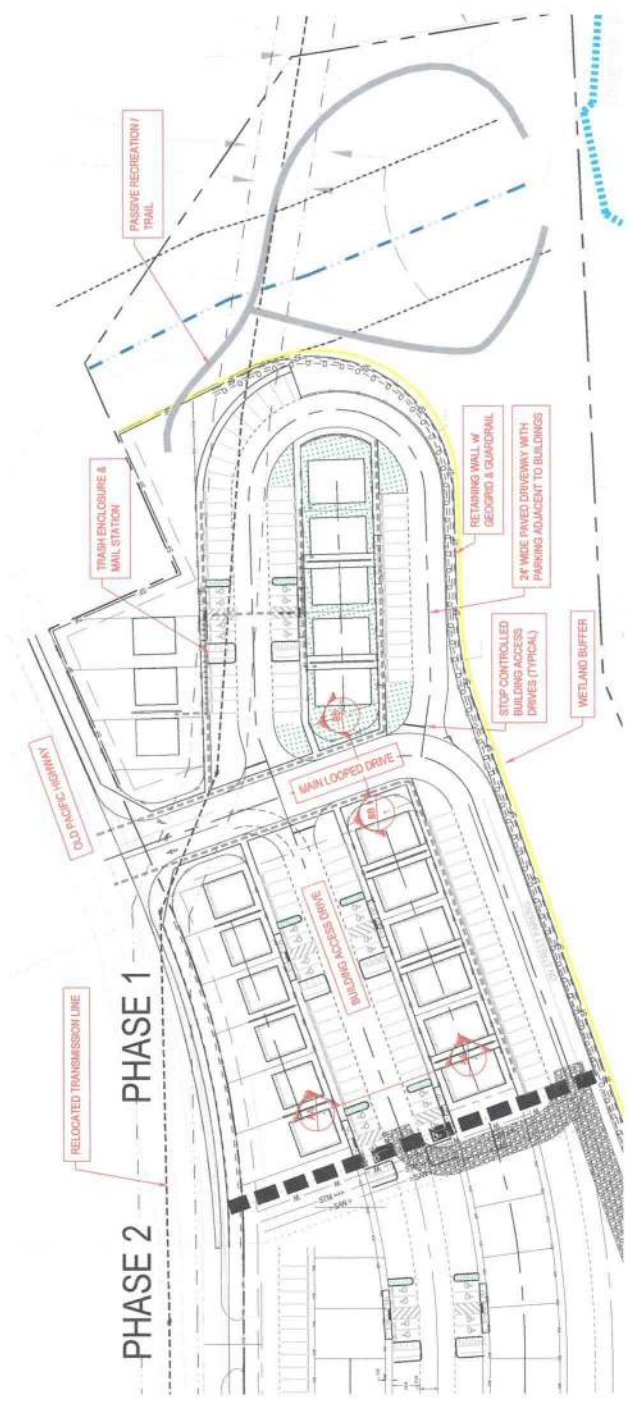


FIGURE 1a

PRELIMINARY

- DESIGN CRITERIA:**
- ROAD**
- THE MAXIMUM ROADWAY SLOPE IS 15%.
 - THE RECOMMENDED INTERSECTION SLOPE IS < 5%.
 - THE MIN RADIUS INSIDE RADIUS FOR A FIRE APPARATUS ACCESS ROAD IS 25' AND 45' FOR AN OUTSIDE RADIUS.
- PARKING**
- ACCESSIBLE PARKING IS 2% MAX.
 - THE RECOMMENDED PARKING SLOPE IS < 5%.
 - PARALLEL PARKING AT APPROPRIATE LOCATIONS.
- SIDEWALKS**
- GENERALLY ALL SIDEWALKS ARE INTENDED TO BE WITHIN PUBLIC RIGHT OF WAY.
- SIDEWALK SLOPE CAN MATCH ROAD PER PRIVATE RAMP 5% MAX.
- BUILDING**
- 10' SPACING
 - 8 PLEX AND 4 PLEX WITH FIREWALL
 - INTENDED TO BE THE FOLLOWING TYPES
 - 1. - FOUR STORY WITH GROUND AND THIRD STORY ACCESS
 - 2. - THREE STORY WITH GROUND ACCESS
 - 3. - FOUR STORY WITH GROUND AND SECOND STORY ACCESS



Phase 1 Conceptual Siteplan

THE LOFTS AT KALAMA
KALAMA, WA

PRE-APP SUBMITTAL
Issue Date: 05/11/2021

Duluth, MN | Vancouver, WA
www.windsorengineers.com
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FIGURE 1c (b)

PHASE 2 PHASE 2

FRONTAGE ROAD DRIVEWAY ACCESS POINTS LINED UP WITH CEDAR SPRINGS DEVELOPMENT

TRANSMISSION LINE RELOCATED TO NORTH SIDE OF ROAD

FRONTAGE DRIVE WITH PARKING ON BUILDING SIDE ONLY

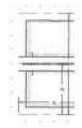
TEMP ACCESS AND UTILITIES BUILT WITH PHASE 1

SANITARY GENERATOR STATION

POTENTIAL INDOOR/OUTDOOR FACILITY WITH OUTER 25% OF BUFFER

STORMWATER OUTFALL

PRELIMINARY



LINE & SYMBOLS
SCALE DRAWING

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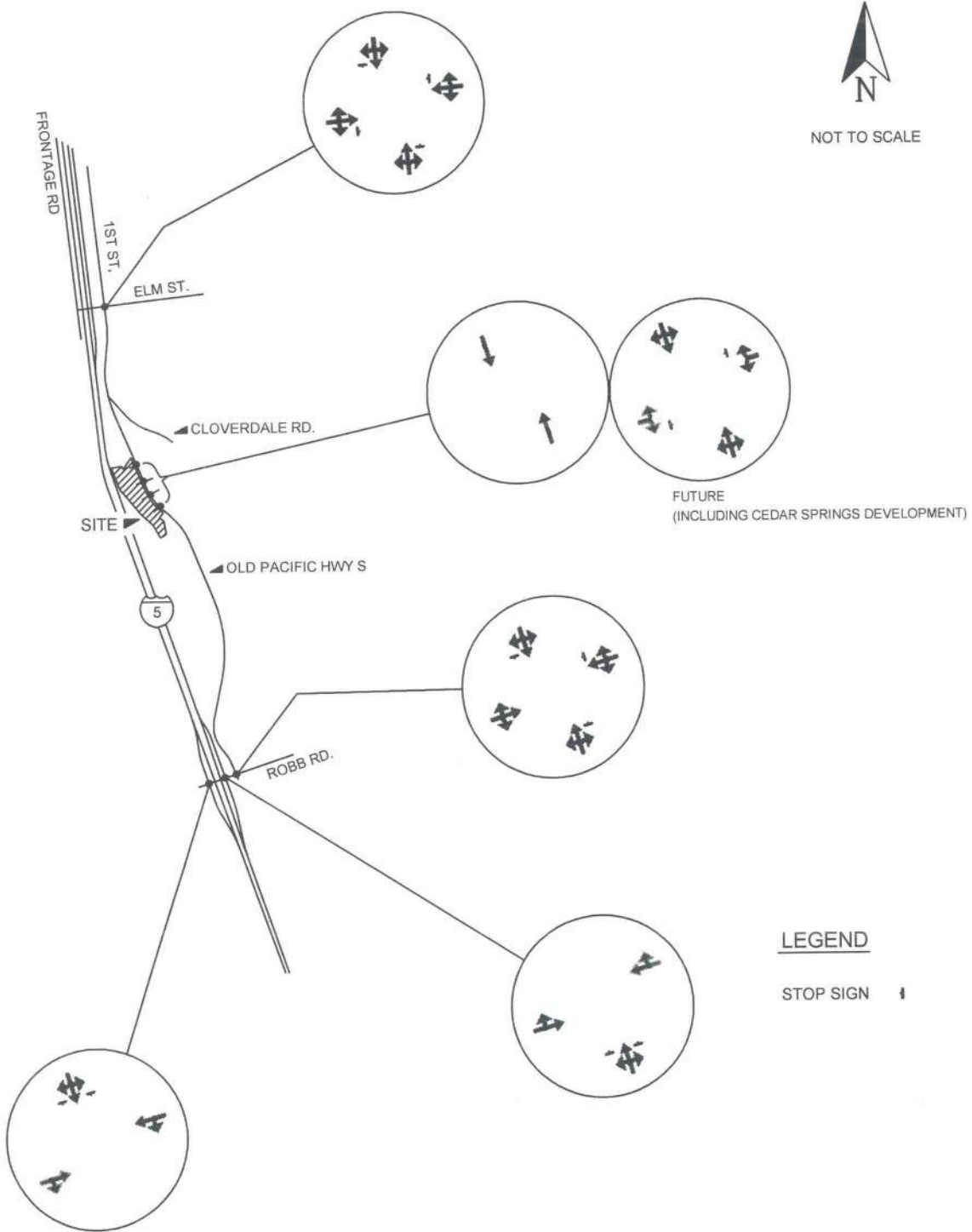
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100% DESIGN

THE LOFTS AT KALAMA
KALAMA, WA

PRE-APP SUBMITTAL
Issue Date: 05/11/2021

Phase 2 Conceptual Siteplan

FIGURE 1c (c)



EXISTING CONDITIONS UNLESS NOTED

LOFTS AT KALAMA

FIGURE 2
LANE CONFIGURATIONS

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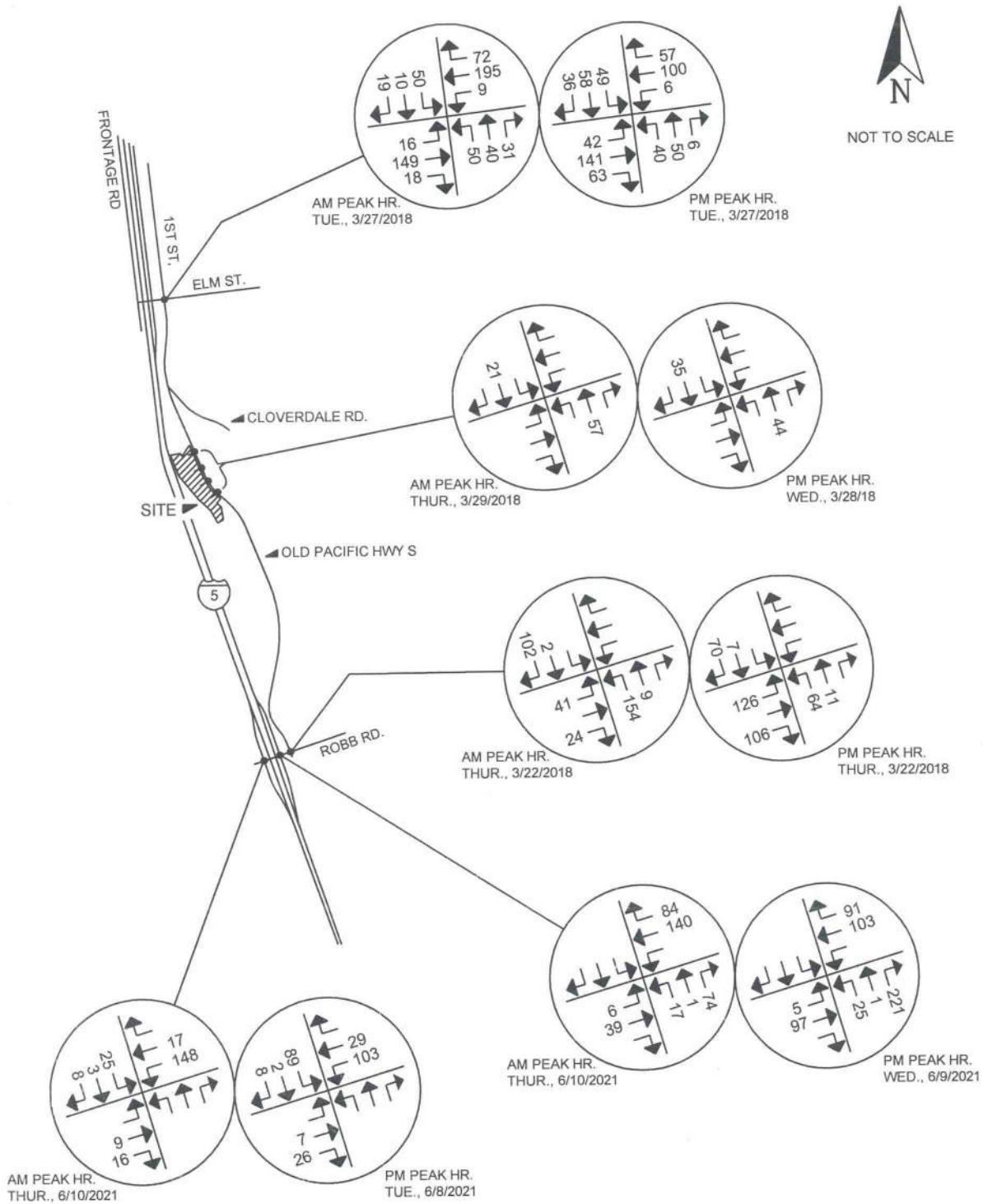
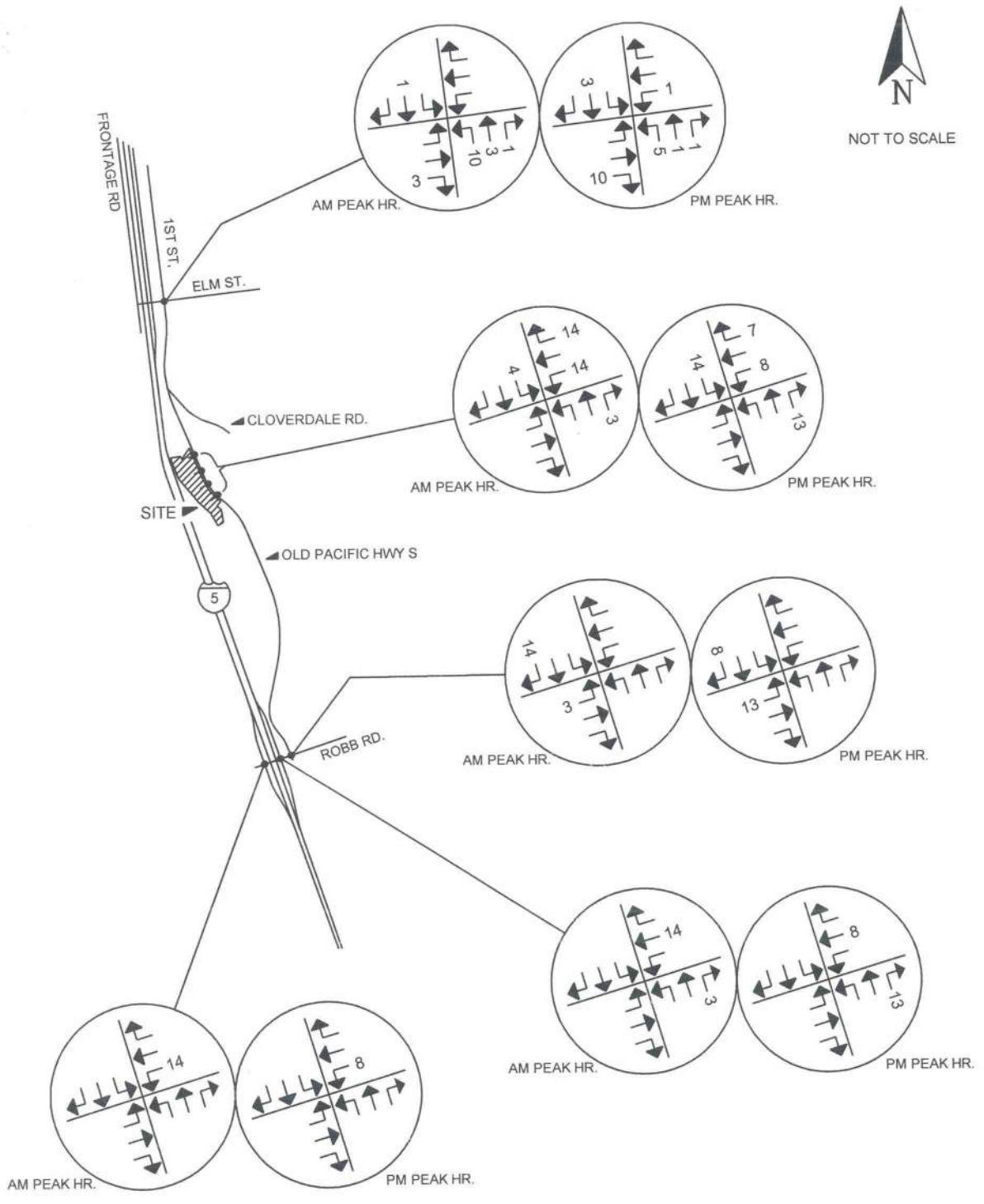


FIGURE 3
EXISTING TRAFFIC VOLUMES

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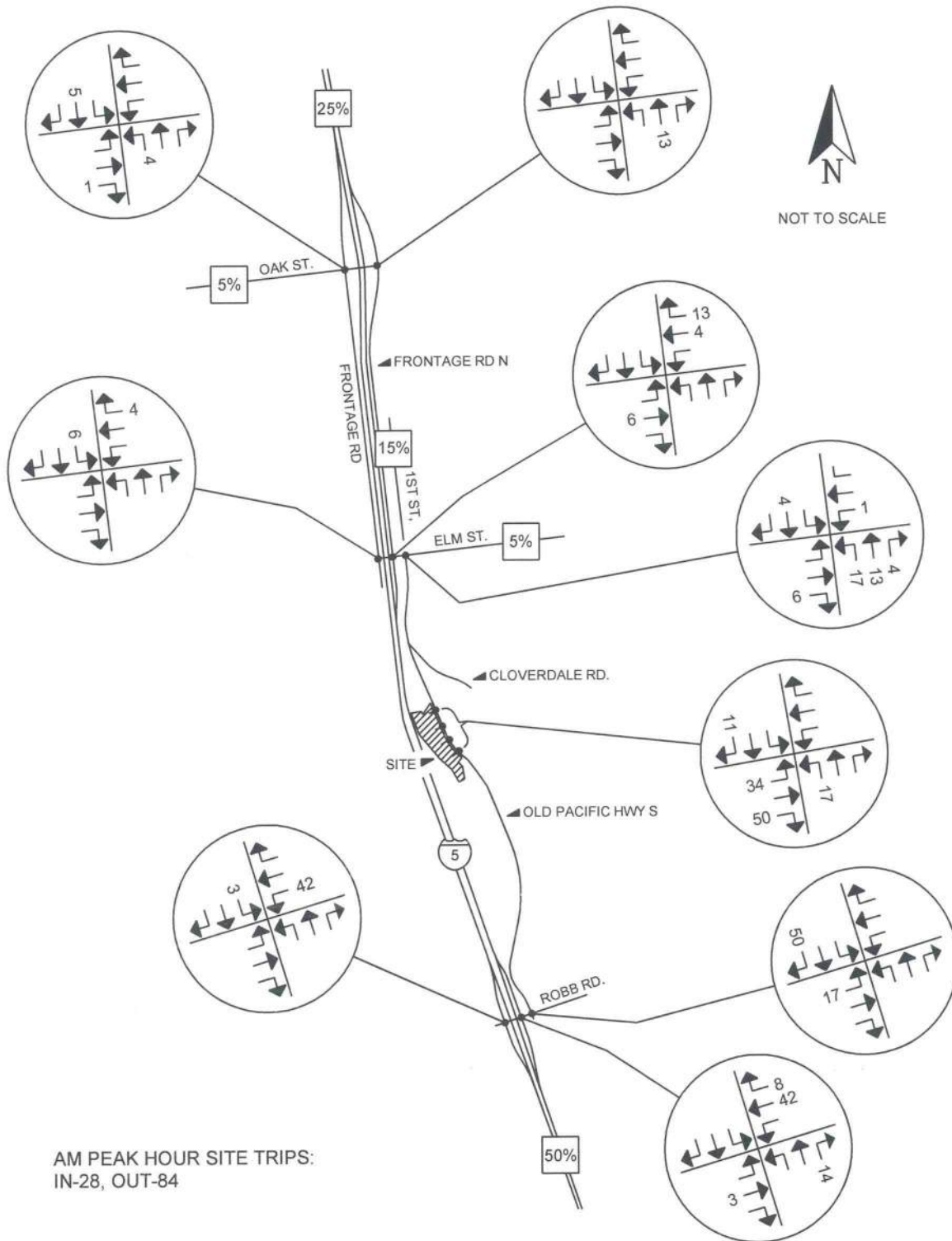


N
NOT TO SCALE

LOFTS AT KALAMA

FIGURE 5
IN-PROCESS TRAFFIC

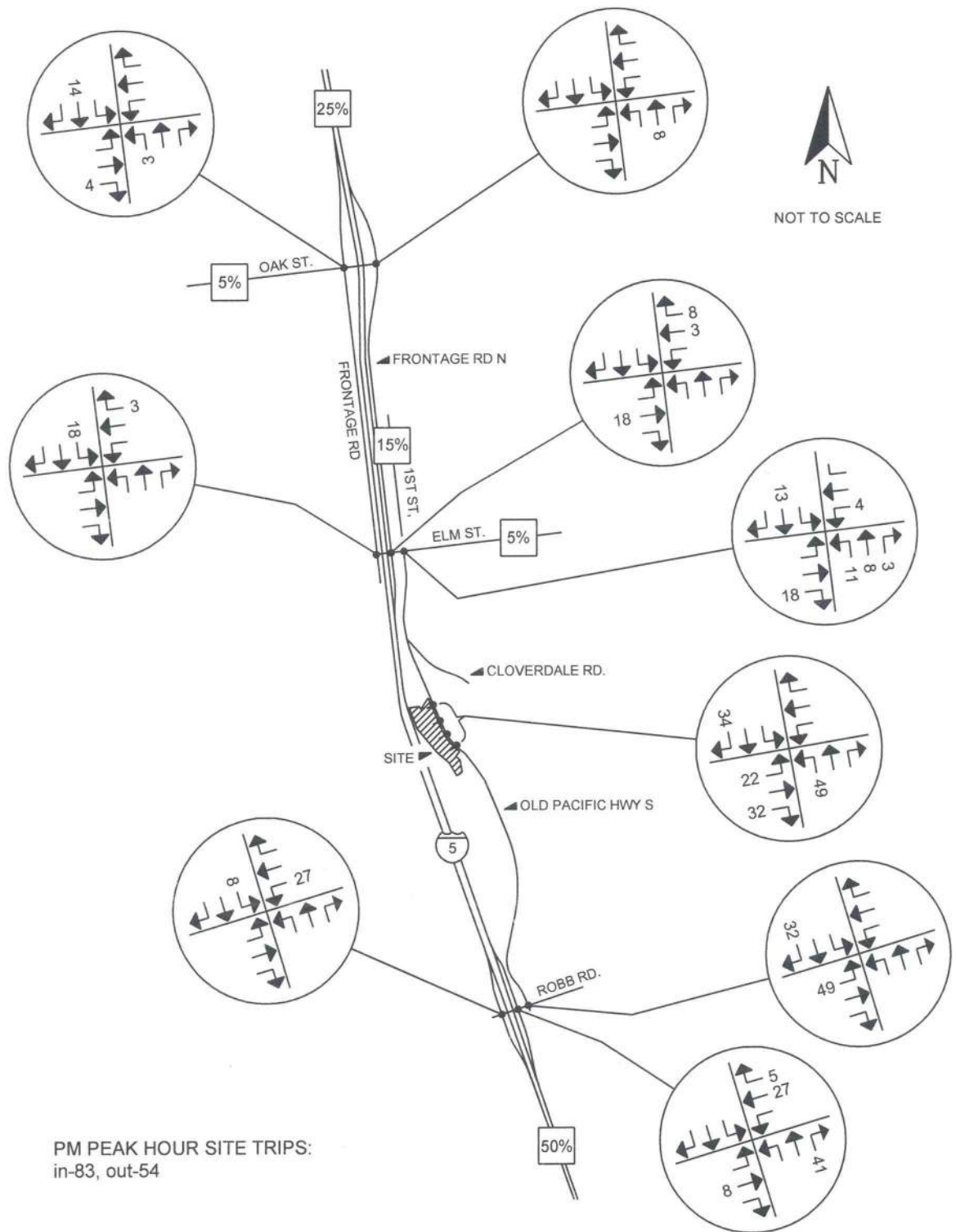
KELLY ENGINEERING
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LOFTS AT KALAMA

FIGURE 7a
**SITE TRAFFIC DISTRIBUTION/
 ASSIGNMENT, AM PEAK HOUR**

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LOFTS AT KALAMA

FIGURE 7b
**SITE TRAFFIC DISTRIBUTION/
 ASSIGNMENT, PM PEAK HOUR**

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APPENDIX A
RAW TRAFFIC COUNT DATA

INTERSECTION TURN MOVEMENT SURVEY ELM STREET & FIRST STREET

DATE OF COUNT: 3/27/2018, 07:00-09:00
 DAY OF WEEK: TUE.
 WEATHER: RAIN
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	3	2	4	0	4	2	3	2	0	1	3	1	25
07:05-07:10	0	1	3	1	8	4	7	4	0	0	5	0	33
07:10-07:15	1	0	0	0	13	1	4	3	0	0	1	0	23
07:15-07:20	1	1	0	0	6	3	4	2	0	2	2	0	21
07:20-07:25	1	2	1	0	12	2	0	5	0	0	7	0	30
07:25-07:30	1	3	2	1	12	5	7	3	2	0	7	1	44
07:30-07:35	3	1	3	1	16	3	7	0	1	1	7	0	43
07:35-07:40	0	1	4	0	12	1	9	3	0	3	9	1	43
07:40-07:45	4	0	2	0	12	3	6	3	1	0	8	1	40
07:45-07:50	2	0	0	0	11	3	4	2	0	1	17	3	43
07:50-07:55	5	0	2	0	16	2	4	4	1	0	11	1	46
07:55-08:00	3	3	1	0	14	3	5	2	2	1	10	0	44
08:00-08:05	2	0	1	0	11	4	3	3	6	0	16	3	49
08:05-08:10	7	1	0	2	15	6	1	6	5	1	18	1	63
08:10-08:15	8	1	2	0	16	7	3	4	5	4	13	1	64
08:15-08:20	6	0	2	3	23	9	2	4	3	3	20	2	77
08:20-08:25	4	2	1	1	21	13	3	3	6	3	16	2	75
08:25-08:30	5	0	2	0	22	11	4	3	1	0	10	2	60
08:30-08:35	4	2	2	3	22	10	6	3	1	0	1	1	55
08:35-08:40	2	1	1	1	14	2	1	1	0	1	1	1	26
08:40-08:45	2	1	0	0	9	2	4	3	0	0	6	3	30
08:45-08:50	3	0	1	0	8	1	4	2	0	0	4	2	25
08:50-08:55	4	0	2	1	8	2	2	4	1	1	3	1	29
08:55-09:00	2	1	1	0	9	3	1	3	0	1	2	1	24
Peak Hour Total	50	10	19	9	195	72	50	40	31	16	149	18	659
% Trucks	0	0	0	11	1	0	0	0	0	6	0	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 07:35-08:35
 PHF Intersection: 0.77

INTERSECTION TURN MOVEMENT SURVEY ELM STREET & FIRST STREET

DATE OF COUNT: 3/27/2018, 16:00-18:00
 DAY OF WEEK: TUE.
 WEATHER: CLOUDY, DRY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
16:00-16:05	5	4	3	1	2	12	2	5	0	1	12	5	52
16:05-16:10	4	1	3	1	7	9	4	3	0	7	9	8	56
16:10-16:15	5	5	0	0	7	7	0	4	1	6	16	3	54
16:15-16:20	4	5	2	1	8	5	2	6	1	2	14	6	56
16:20-16:25	7	5	4	1	5	2	0	1	0	3	14	2	44
16:25-16:30	7	6	0	0	3	6	3	3	2	4	7	4	45
16:30-16:35	2	2	3	1	4	5	3	3	1	3	15	5	47
16:35-16:40	8	1	4	1	6	8	1	4	0	2	9	7	51
16:40-16:45	5	5	3	0	7	6	6	1	0	5	10	6	54
16:45-16:50	6	6	1	0	9	3	2	4	0	4	15	4	54
16:50-16:55	3	4	0	1	7	3	5	4	0	5	6	7	45
16:55-17:00	3	4	3	0	4	3	2	6	0	6	9	5	45
17:00-17:05	0	4	4	1	10	4	0	5	1	2	13	5	49
17:05-17:10	6	4	7	1	8	3	2	7	0	2	10	2	52
17:10-17:15	4	6	1	1	7	4	1	4	1	4	11	8	52
17:15-17:20	6	5	1	0	12	6	5	3	2	3	10	2	55
17:20-17:25	5	9	5	1	6	3	4	7	0	2	11	6	59
17:25-17:30	4	3	1	0	11	4	5	6	1	4	16	7	62
17:30-17:35	2	2	5	1	10	10	6	0	1	1	19	7	64
17:35-17:40	5	6	5	0	9	8	2	3	0	4	11	4	57
17:40-17:45	7	3	2	1	8	6	0	4	0	2	14	1	48
17:45-17:50	4	4	3	1	5	5	2	2	0	3	13	3	45
17:50-17:55	5	3	1	0	7	4	1	2	2	2	10	4	41
17:55-18:00	6	3	2	0	6	3	0	3	0	3	10	5	41
Peak Hour Total	49	58	36	6	100	57	40	50	6	42	141	63	648
% Trucks	0	0	0	17	0	0	0	0	0	0	1	0	
Peds	0	5	0	0	2	0	0	1	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 16:40-17:40

PHF Intersection: 0.88

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**OLD PACIFIC HWY S SURVEY
SITE ACCESS & OLD PACIFIC HWY S**

DATE OF COUNT: 3/29/2018, 07:00-09:00
 DAY OF WEEK: THUR.
 WEATHER: CLOUDY, DRY
 COUNTER: DSK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	0	1	0	0	0	0	0	2	0	0	0	0	3
07:05-07:10	0	3	0	0	0	0	0	2	0	0	0	0	5
07:10-07:15	0	2	0	0	0	0	0	1	0	0	0	0	3
07:15-07:20	0	1	0	0	0	0	0	3	0	0	0	0	4
07:20-07:25	0	2	0	0	0	0	0	3	0	0	0	0	5
07:25-07:30	0	2	0	0	0	0	0	5	0	0	0	0	7
07:30-07:35	0	1	0	0	0	0	0	4	0	0	0	0	5
07:35-07:40	0	0	0	0	0	0	0	1	0	0	0	0	1
07:40-07:45	0	2	0	0	0	0	0	5	0	0	0	0	7
07:45-07:50	0	2	0	0	0	0	0	2	0	0	0	0	4
07:50-07:55	0	4	0	0	0	0	0	1	0	0	0	0	5
07:55-08:00	0	4	0	0	0	0	0	2	0	0	0	0	6
08:00-08:05	0	2	0	0	0	0	0	3	0	0	0	0	5
08:05-08:10	0	1	0	0	0	0	0	4	0	0	0	0	5
08:10-08:15	0	0	0	0	0	0	0	4	0	0	0	0	4
08:15-08:20	0	2	0	0	0	0	0	2	0	0	0	0	4
08:20-08:25	0	2	0	0	0	0	0	0	0	0	0	0	2
08:25-08:30	0	0	0	0	0	0	0	5	0	0	0	0	5
08:30-08:35	0	1	0	0	0	0	0	3	0	0	0	0	4
08:35-08:40	0	1	0	0	0	0	0	5	0	0	0	0	6
08:40-08:45	0	1	0	0	0	0	0	5	0	0	0	0	6
08:45-08:50	0	2	0	0	0	0	0	7	0	0	0	0	9
08:50-08:55	0	4	0	0	0	0	0	10	0	0	0	0	14
08:55-09:00	0	5	0	0	0	0	0	9	0	0	0	0	14
Peak Hour Total	0	21	0	0	0	0	0	57	0	0	0	0	78
% Trucks	0	5	0	0	0	0	0	3	0	0	0	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 08:00-09:00

**OLD PACIFIC HWY S SURVEY
SITE ACCESS & OLD PACIFIC HWY S**

DATE OF COUNT: 3/28/18, 16:00-18:00
 DAY OF WEEK: WED.
 WEATHER: RAIN
 COUNTER: DSK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
16:00-16:05	0	6	0	0	0	0	0	7	0	0	0	0	13
16:05-16:10	0	1	0	0	0	0	0	9	0	0	0	0	10
16:10-16:15	0	4	0	0	0	0	0	3	0	0	0	0	7
16:15-16:20	0	4	0	0	0	0	0	2	0	0	0	0	6
16:20-16:25	0	3	0	0	0	0	0	4	0	0	0	0	7
16:25-16:30	0	5	0	0	0	0	0	2	0	0	0	0	7
16:30-16:35	0	1	0	0	0	0	0	6	0	0	0	0	7
16:35-16:40	0	2	0	0	0	0	0	5	0	0	0	0	7
16:40-16:45	0	5	0	0	0	0	0	1	0	0	0	0	6
16:45-16:50	0	0	0	0	0	0	0	3	0	0	0	0	3
16:50-16:55	0	2	0	0	0	0	0	0	0	0	0	0	2
16:55-17:00	0	2	0	0	0	0	0	2	0	0	0	0	4
17:00-17:05	0	4	0	0	0	0	0	2	0	0	0	0	6
17:05-17:10	0	4	0	0	0	0	0	5	0	0	0	0	9
17:10-17:15	0	0	0	0	0	0	0	3	0	0	0	0	3
17:15-17:20	0	3	0	0	0	0	0	1	0	0	0	0	4
17:20-17:25	0	7	0	0	0	0	0	2	0	0	0	0	9
17:25-17:30	0	7	0	0	0	0	0	3	0	0	0	0	10
17:30-17:35	0	2	0	0	0	0	0	1	0	0	0	0	3
17:35-17:40	0	1	0	0	0	0	0	2	0	0	0	0	3
17:40-17:45	0	3	0	0	0	0	0	3	0	0	0	0	6
17:45-17:50	0	1	0	0	0	0	0	2	0	0	0	0	3
17:50-17:55	0	2	0	0	0	0	0	1	0	0	0	0	3
17:55-18:00	0	1	0	0	0	0	0	0	0	0	0	0	1
Peak Hour Total	0	35	0	0	0	0	0	44	0	0	0	0	79
% Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0	0	0	0
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	0

PEAK HOUR: 16:00-17:00

**INTERSECTION TURN MOVEMENT SURVEY
OLD PACIFIC HIGHWAY & ROBB ROAD/TODD ROAD**

DATE OF COUNT: 3/22/2018, 07:00-09:00
 DAY OF WEEK: THUR.
 WEATHER: CLOUDY, DRY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	0	1	9	0	0	0	7	0	0	4	0	1	22
07:05-07:10	0	0	23	0	0	0	9	0	0	2	0	2	36
07:10-07:15	0	0	12	0	0	0	7	1	0	2	0	5	27
07:15-07:20	0	0	7	0	0	0	5	0	0	3	0	0	15
07:20-07:25	0	0	7	0	0	0	18	1	0	3	0	3	32
07:25-07:30	0	0	6	0	0	0	11	1	0	3	0	1	22
07:30-07:35	0	0	11	0	0	0	14	1	0	1	0	2	29
07:35-07:40	0	0	7	0	0	0	11	1	0	2	0	1	22
07:40-07:45	0	0	11	0	0	0	15	0	0	3	0	5	34
07:45-07:50	0	0	6	0	0	0	16	1	0	3	0	2	28
07:50-07:55	0	0	5	0	0	0	12	0	0	1	0	1	19
07:55-08:00	0	0	9	0	0	0	17	0	0	5	0	3	34
08:00-08:05	0	0	8	0	0	0	9	1	0	2	0	3	23
08:05-08:10	0	0	9	0	0	0	7	0	0	3	0	0	19
08:10-08:15	0	2	14	0	0	0	9	2	0	8	0	2	37
08:15-08:20	0	0	9	0	0	0	15	1	0	7	0	1	33
08:20-08:25	0	0	7	0	0	0	9	0	0	5	0	6	27
08:25-08:30	0	0	7	0	0	0	5	1	0	2	0	3	18
08:30-08:35	0	1	6	0	0	0	3	0	0	4	0	3	17
08:35-08:40	0	1	5	0	0	0	9	0	0	7	0	3	25
08:40-08:45	0	1	6	0	0	0	2	0	0	3	0	4	16
08:45-08:50	0	0	7	0	0	0	4	1	0	4	0	2	18
08:50-08:55	0	0	9	0	0	0	5	0	0	2	0	1	17
08:55-09:00	0	1	8	0	0	0	3	0	0	1	0	3	16
Peak Hour Total	0	2	102	0	0	0	154	9	0	41	0	24	332
% Trucks	0	0	11	0	0	0	1	11	0	15	0	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 07:20-08:20

PHF Intersection: 0.93

**INTERSECTION TURN MOVEMENT SURVEY
OLD PACIFIC HIGHWAY & ROBB ROAD/TODD ROAD**

DATE OF COUNT: 3/22/2018, 16:00-18:00
 DAY OF WEEK: THUR.
 WEATHER: SUNNY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
16:00-16:05	0	1	8	0	0	0	13	1	0	10	0	8	41
16:05-16:10	0	3	8	0	0	0	10	2	0	12	0	4	39
16:10-16:15	0	1	8	0	0	0	3	1	0	13	0	10	36
16:15-16:20	0	0	3	0	0	0	6	2	0	8	0	11	30
16:20-16:25	0	0	7	0	0	0	8	1	0	19	0	8	43
16:25-16:30	0	0	1	0	0	0	2	0	0	15	0	9	27
16:30-16:35	0	0	5	0	0	0	6	0	0	12	0	15	38
16:35-16:40	0	0	7	0	0	0	2	0	0	5	0	14	28
16:40-16:45	0	2	6	0	0	0	3	1	0	15	0	6	33
16:45-16:50	0	0	4	0	0	0	6	0	0	5	0	8	23
16:50-16:55	0	0	4	0	0	0	3	2	0	5	0	6	20
16:55-17:00	0	0	9	0	0	0	2	1	0	7	0	7	26
17:00-17:05	1	0	6	0	0	0	6	0	0	11	0	10	34
17:05-17:10	0	0	6	0	0	0	2	0	0	11	0	4	23
17:10-17:15	0	1	9	0	0	0	4	1	0	11	0	9	35
17:15-17:20	0	0	11	0	0	0	5	2	0	12	0	11	41
17:20-17:25	0	2	10	0	0	0	7	0	0	12	0	8	39
17:25-17:30	0	2	5	0	0	0	10	0	0	6	0	9	32
17:30-17:35	0	0	10	0	0	0	3	0	0	7	0	7	27
17:35-17:40	0	1	8	0	0	0	3	1	0	10	0	15	38
17:40-17:45	0	0	8	0	0	0	2	1	0	8	0	8	27
17:45-17:50	0	0	5	0	0	0	2	0	0	7	0	9	23
17:50-17:55	0	1	7	0	0	0	4	0	0	7	0	5	24
17:55-18:00	0	0	7	0	0	0	5	1	0	9	0	7	29
Peak Hour Total	0	7	70	0	0	0	64	11	0	126	0	106	384
% Trucks	0	0	4	0	0	0	3	0	0	6	0	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 16:00-17:00

PHF Intersection: 0.83

INTERSECTION TURN MOVEMENT SURVEY I-5 NB RAMPS & ROBB ROAD

DATE OF COUNT: 6/10/2021, 07:00-09:00
 DAY OF WEEK: THUR.
 WEATHER: CLOUDY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	0	0	0	0	8	7	0	0	0	1	1	0	17
07:05-07:10	0	0	0	0	10	2	0	0	1	0	2	0	15
07:10-07:15	0	0	0	0	18	10	1	0	1	0	3	0	33
07:15-07:20	0	0	0	0	11	5	1	0	2	0	2	0	21
07:20-07:25	0	0	0	0	13	4	0	0	3	0	3	0	23
07:25-07:30	0	0	0	0	16	4	2	0	7	1	1	0	31
07:30-07:35	0	0	0	0	16	3	1	0	9	0	2	0	31
07:35-07:40	0	0	0	0	20	17	1	0	5	2	5	0	50
07:40-07:45	0	0	0	0	8	6	0	0	2	3	0	0	19
07:45-07:50	0	0	0	0	4	9	3	0	6	0	4	0	26
07:50-07:55	0	0	0	0	10	8	1	0	11	0	1	0	31
07:55-08:00	0	0	0	0	17	10	2	1	7	0	4	0	41
08:00-08:05	0	0	0	0	12	6	0	0	1	0	5	0	24
08:05-08:10	0	0	0	0	9	6	2	0	5	0	2	0	24
08:10-08:15	0	0	0	0	13	5	0	0	5	0	5	0	28
08:15-08:20	0	0	0	0	7	5	2	0	8	0	7	0	29
08:20-08:25	0	0	0	0	8	5	3	0	8	0	3	0	27
08:25-08:30	0	0	0	0	6	7	1	0	4	1	4	0	23
08:30-08:35	0	0	0	0	4	8	3	0	8	0	5	0	28
08:35-08:40	0	0	0	0	12	9	1	0	6	0	4	0	32
08:40-08:45	0	0	0	0	11	13	2	0	4	0	2	0	32
08:45-08:50	0	0	0	0	8	9	1	0	5	0	1	0	24
08:50-08:55	0	0	0	0	7	8	2	0	2	0	3	0	22
08:55-09:00	0	0	0	0	7	4	0	0	3	0	1	0	15
Peak Hour Total	0	0	0	0	140	84	17	1	74	6	39	0	361
% Trucks	0	0	0	0	6	7	18	0	8	17	5	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 07:25-08:25

PHF Intersection: 0.81

INTERSECTION TURN MOVEMENT SURVEY I-5 NB RAMPS & ROBB ROAD

DATE OF COUNT: 6/9/2021, 16:00-18:00
 DAY OF WEEK: WED.
 WEATHER: CLOUDY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
16:00-16:05	0	0	0	0	10	7	3	0	9	0	8	0	37
16:05-16:10	0	0	0	0	9	6	4	0	7	0	9	0	35
16:15-16:20	0	0	0	0	8	5	2	0	10	2	11	0	38
16:10-16:15	0	0	0	0	10	5	5	0	6	0	10	0	36
16:20-16:25	0	0	0	0	8	5	3	0	21	1	9	0	47
16:25-16:30	0	0	0	0	5	6	4	0	21	1	7	0	44
16:30-16:35	0	0	0	0	9	12	1	1	20	1	12	0	56
16:35-16:40	0	0	0	0	9	6	2	0	25	0	8	0	50
16:40-16:45	0	0	0	0	10	7	4	0	12	0	6	0	39
16:45-16:50	0	0	0	0	5	2	2	0	29	0	6	0	44
16:50-16:55	0	0	0	0	5	9	3	0	16	1	6	0	40
16:55-17:00	0	0	0	0	9	11	2	0	20	0	9	0	51
17:00-17:05	0	0	0	0	13	6	1	0	13	0	7	0	40
17:05-17:10	0	0	0	0	12	6	0	0	14	1	4	0	37
17:10-17:15	0	0	0	0	9	10	0	0	15	0	11	0	45
17:15-17:20	0	0	0	0	9	11	3	0	15	0	12	0	50
17:20-17:25	0	0	0	0	6	7	4	0	16	2	6	0	41
17:25-17:30	0	0	0	0	7	6	2	0	13	0	5	0	33
17:30-17:35	0	0	0	0	4	4	4	1	15	0	8	0	36
17:35-17:40	0	0	0	0	12	7	4	0	10	0	4	0	37
17:40-17:45	0	0	0	0	7	4	2	1	14	0	9	0	37
17:45-17:50	0	0	0	0	6	2	0	0	10	1	5	0	24
17:50-17:55	0	0	0	0	5	5	3	0	9	0	7	0	29
17:55-18:00	0	0	0	0	8	7	2	0	11	0	10	0	38
Peak Hour Total	0	0	0	0	103	91	25	1	221	5	97	0	543
% Trucks	0	0	0	0	7	3	0	0	2	20	7	0	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 16:20-17:20

PHF Intersection: 0.91

KELLY ENGINEERING

INTERSECTION TURN MOVEMENT SURVEY I-5 SB RAMPS & ROBB ROAD

DATE OF COUNT: 6/10/2021, 07:00-09:00
 DAY OF WEEK: THUR.
 WEATHER: CLOUDY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
07:00-07:05	3	0	2	8	0	0	0	0	0	0	1	1	15
07:05-07:10	2	0	2	10	0	0	0	0	0	0	0	2	16
07:10-07:15	2	1	0	16	2	0	0	0	0	0	0	0	21
07:15-07:20	0	1	0	11	3	0	0	0	0	0	2	5	22
07:20-07:25	2	0	0	14	1	0	0	0	0	0	1	1	19
07:25-07:30	1	0	1	13	2	0	0	0	0	0	1	2	20
07:30-07:35	2	0	1	15	1	0	0	0	0	0	0	0	19
07:35-07:40	4	0	0	16	1	0	0	0	0	0	1	2	24
07:40-07:45	0	0	0	9	0	0	0	0	0	0	1	0	10
07:45-07:50	3	0	3	4	3	0	0	0	0	0	2	1	16
07:50-07:55	1	0	1	11	2	0	0	0	0	0	0	1	16
07:55-08:00	3	1	0	17	0	0	0	0	0	0	0	2	23
08:00-08:05	5	0	0	12	2	0	0	0	0	0	1	0	20
08:05-08:10	2	0	0	9	1	0	0	0	0	0	0	1	13
08:10-08:15	5	0	0	13	2	0	0	0	0	0	0	3	23
08:15-08:20	7	1	0	7	1	0	0	0	0	0	0	1	17
08:20-08:25	4	0	0	6	3	0	0	0	0	0	1	0	14
08:25-08:30	2	1	0	7	1	0	0	0	0	0	2	1	14
08:30-08:35	5	0	0	4	3	0	0	0	0	0	0	0	12
08:35-08:40	4	0	0	12	1	0	0	0	0	0	0	1	18
08:40-08:45	1	1	0	10	2	0	0	0	0	0	1	1	16
08:45-08:50	2	0	0	5	2	0	0	0	0	0	0	0	9
08:50-08:55	0	0	0	6	1	0	0	0	0	0	0	1	8
08:55-09:00	2	0	0	9	0	0	0	0	0	0	1	0	12
Peak Hour Total	25	3	8	148	17	0	0	0	0	0	9	16	226
% Trucks	12	33	50	5	18	0	0	0	0	0	11	31	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 07:05-08:05
 PHF Intersection: 0.90

**INTERSECTION TURN MOVEMENT SURVEY
I-5 SB RAMPS & ROBB ROAD**

DATE OF COUNT: 6/8/2021, 16:00-18:00
 DAY OF WEEK: TUE.
 WEATHER: CLOUDY
 COUNTER: KAK

Time Period From – To	FROM NORTH			FROM EAST			FROM SOUTH			FROM WEST			TOTAL
	L	T	R	L	T	R	L	T	R	L	T	R	
16:00-16:05	7	0	0	7	2	0	0	0	0	0	1	1	18
16:05-16:10	9	0	0	8	3	0	0	0	0	0	0	0	20
16:10-16:15	13	1	0	9	2	0	0	0	0	0	0	2	27
16:15-16:20	8	0	2	11	5	0	0	0	0	0	0	2	28
16:20-16:25	10	0	1	9	3	0	0	0	0	0	1	4	28
16:25-16:30	6	0	1	3	4	0	0	0	0	0	1	4	19
16:30-16:35	10	0	0	8	2	0	0	0	0	0	0	5	25
16:35-16:40	8	0	0	8	2	0	0	0	0	0	0	1	19
16:40-16:45	6	0	1	9	3	0	0	0	0	0	0	2	21
16:45-16:50	5	0	0	6	3	0	0	0	0	0	1	1	16
16:50-16:55	6	1	0	7	2	0	0	0	0	0	0	0	16
16:55-17:00	9	0	0	7	2	0	0	0	0	0	1	1	20
17:00-17:05	6	0	2	13	1	0	0	0	0	0	3	3	28
17:05-17:10	2	0	1	13	0	0	0	0	0	0	0	1	17
17:10-17:15	11	0	1	8	1	0	0	0	0	0	0	0	21
17:15-17:20	11	0	1	9	3	0	0	0	0	0	0	0	24
17:20-17:25	6	0	0	5	4	0	0	0	0	0	0	1	16
17:25-17:30	5	0	0	7	2	0	0	0	0	0	1	2	17
17:30-17:35	7	1	0	4	4	0	0	0	0	0	1	5	22
17:35-17:40	4	0	0	12	4	0	0	0	0	0	0	0	20
17:40-17:45	8	0	0	6	2	0	0	0	0	0	0	0	16
17:45-17:50	5	0	0	8	3	0	0	0	0	0	0	2	18
17:50-17:55	7	0	0	8	1	0	0	0	0	0	0	0	16
17:55-18:00	3	0	0	5	4	0	0	0	0	0	0	0	12
Peak Hour Total	89	2	8	103	29	0	0	0	0	0	7	26	264
% Trucks	7	50	0	7	0	0	0	0	0	0	0	4	
Peds	0	0	0	0	0	0	0	0	0	0	0	0	
Bikes	0	0	0	0	0	0	0	0	0	0	0	0	

PEAK HOUR: 16:10-17:10
 PHF Intersection: 0.80

APPENDIX B
IN-PROCESS TRAFFIC

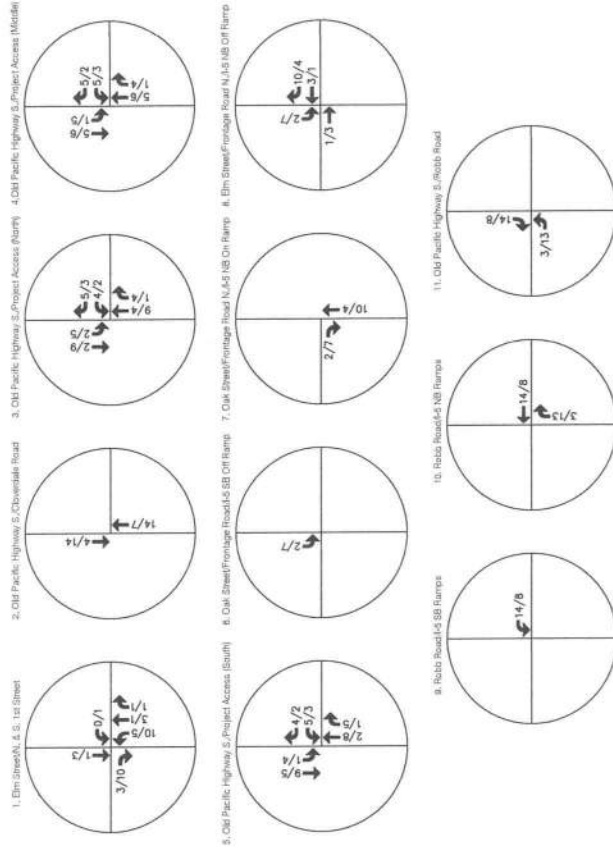
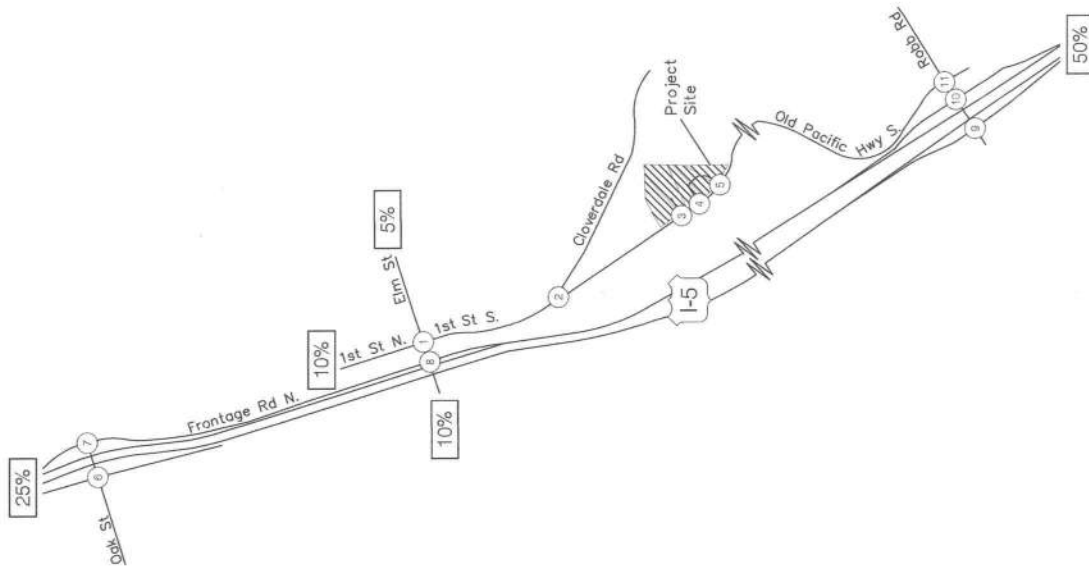


FIGURE 6
Trip Distribution and Assignment
Traffic Volumes

LEGEND
A.M./P.M. Peak Hour
100/128
Traffic Volumes
Peak Hour Trip Distribution
10%

NOT TO SCALE

**APPENDIX C
COLLISION DATA**

CITY STREETS

ELM ST @ 1st ST

COUNTY ROADS

TODD RD - *south leg* (CO RD #32840, MP 0.590 - 0.630) @ ROBB RD (CO RD # 33320, MP 0.000 - 0.020) - *No Reported Crashes*
OLD PACIFIC HWY - *north leg* (CO RD #33950, MP 1.960 - 1.980) @ ROBB RD (CO RD # 33320, MP 0.000 - 0.020) - *No Reported Crashes*

STATE ROUTES

005LX02770 (aka Robb Rd, MP 0.000 - 0.020) @ SB SR 5 ON/OFF-RAMPS
005R102800 (MP 0.26 - 0.28) @ ROBB RD - *No Reported Crashes*
005S102722 (MP 0.00 - 0.02) @ ROBB RD - *No Reported Crashes*
005LX02770 (aka Robb Rd, MP 0.08 - 0.10) @ NB SR 5 ON/OFF-RAMPS
005P102740 (MP 0.33 - 0.35) @ ROBB RD - *No Reported Crashes*
005Q102822 (MP 0.00 - 0.02) @ ROBB RD - *No Reported Crashes*

OFFICER REPORTED CRASHES THAT OCCURRED AT OR IN THE VICINITY OF MULTIPLE INTERSECTIONS IN THE CITY OF KALAMA & COWLITZ COUNTY

01/01/2018 - available 2021 See 2nd tab below for road information & interchange drawing for reference

Under 23 U.S. Code § 148 and 23 U.S. Code § 409, safety data, reports, surveys, schedules, lists compiled or collected for the purpose of identifying, evaluating, or planning the safety enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossings are not subject to discovery or admitted into evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

JURISDICTION	COUNTY	CITY	PRIMARY TRAFFICWAY	MILEPOST	A / B	BLOCK NUMBER	INTERSECTING TRAFFICWAY	CO ONLY INTERSECTING COUNTY ROAD MILEPOST	DIST FROM REF POINT	MI or FT	COMP DIR FROM REF POINT	REFERENCE POINT NAME
City Street	Cowlitz	Kalama	ELM ST			0	S 1ST ST					
City Street	Cowlitz	Kalama	ELM ST			100			240	F	E	S 1ST ST
City Street	Cowlitz	Kalama	N 1ST ST			0	ELM ST					
City Street	Cowlitz	Kalama	N 1ST ST			101	ELM ST					
City Street	Cowlitz	Kalama	N 1ST ST			100			76	F	N	S 1ST ST
City Street	Cowlitz	Kalama	S 1ST ST			100			100	F	N	ELM ST
State Route	Cowlitz		005LX02770	0.00								
State Route	Cowlitz		005LX02770	0.10								

SR ONLY HISTORY / SUSPENSE IND	REPORT NUMBER	DATE	TIME	MOST SEVERE INJURY TYPE	# I N J U R I E S	# F A T A L I T I E S	# V E H I C L E S	# P E D E S T R I A N S	# B I K E S	VEHICLE 1 TYPE
No	E901014	01/09/2019	18:10	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	EA85661	11/27/2020	13:50	No Apparent Injury	0	0	5	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	E979162	11/04/2019	14:47	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	EB23242	04/19/2021	14:25	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	E950996	07/24/2019	14:40	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	E760976	01/19/2018	10:42	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	EB10244	02/26/2021	10:52	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb
No	E933438	06/23/2019	11:30	No Apparent Injury	0	0	2	0	0	Pickup,Panel Truck or Vanette under 10,000 lb

VEHICLE 2 TYPE	JUNCTION RELATIONSHIP	WEATHER	ROADWAY SURFACE CONDITION	LIGHTING CONDITION
Pickup,Panel Truck or Vanette under 10,000 lb	At Intersection and Related	Overcast	Wet	Dark-Street Lights On
Pickup,Panel Truck or Vanette under 10,000 lb	Not at Intersection and Not Related	Clear	Dry	Daylight
Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight
Pickup,Panel Truck or Vanette under 10,000 lb	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight
Pickup,Panel Truck or Vanette under 10,000 lb	Not at Intersection and Not Related	Clear or Partly Cloudy	Dry	Daylight
Pickup,Panel Truck or Vanette under 10,000 lb	Not at Intersection and Not Related	Clear or Partly Cloudy	Dry	Daylight
Passenger Car	At Intersection and Related	Clear	Dry	Daylight
Pickup,Panel Truck or Vanette under 10,000 lb	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight

FIRST COLLISION TYPE / OBJECT STRUCK	VEHICLE 1 ACTION	VEHICLE 2 ACTION	VEHICLE 1 COMPASS DIRECTION FROM	VEHICLE 1 COMPASS DIRECTION TO
Entering at angle	Going Straight Ahead	Going Straight Ahead	East	West
One parked--one moving	Going Straight Ahead	Legally Parked, Unoccupied	East	West
Entering at angle	Starting in Traffic Lane	Starting in Traffic Lane	South	North
Entering at angle	Going Straight Ahead	Going Straight Ahead	West	East
One car leaving parked position	Backing	Going Straight Ahead	Vehicle Backing	Vehicle Backing
One parked--one moving	Backing	Legally Parked, Unoccupied	Vehicle Backing	Vehicle Backing
From opposite direction - one left turn - one straight	Going Straight Ahead	Making Left Turn	West	East
Entering at angle	Going Straight Ahead	Going Straight Ahead	North	South

VEHICLE 2 COMPASS DIRECTION FROM	VEHICLE 2 COMPASS DIRECTION TO	MV DRIVER CONTRIBUTING CIRCUMSTANCE 1 (UNIT 1)	MV DRIVER CONTRIBUTING CIRCUMSTANCE 1 (UNIT 2)
South	North	Did Not Grant RW to Vehicle	Driver Not Distracted
		None	None
West	East	Did Not Grant RW to Vehicle	None
South	North	None	None
North	South	Improper Backing	None
		Inattention	None
East	South	None	Improper Turn/Merge
West	East	Operating Defective Equipment	Driver Not Distracted

	WA STATE PLANE SOUTH - X 2010 - FORWARD	WA STATE PLANE SOUTH - Y 2010 - FORWARD
FIRST IMPACT LOCATION (City, County & Misc Trafficways - 2010 forward)		
Lane of Primary Trafficway	1044801.99	254755.24
Outside Shoulder of Primary Trafficway	1045032.58	254820.06
Lane of Primary Trafficway	1044801.99	254755.23
Lane of Primary Trafficway	1044801.58	254754.5
Lane of Primary Trafficway	1044677.07	255197.22
Outside Shoulder of Primary Trafficway	1044774.8	254851.45
Lane 1 LX Increasing Milepost (Prior to 2002 Impact Location Code was not lane specific)	1049276.29	244559.97
Lane 1 LX Increasing Milepost (Prior to 2002 Impact Location Code was not lane specific)	1049719.86	244836.69

APPENDIX D
LEVEL OF SERVICE COMPUTER PRINTOUTS

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Elm St. & 1st St.
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	AM Peak Hour		

Project ID Existing

East/West Street: Elm St.

North/South Street: 1st St.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound				
	L	T	R	L	T	R		
Movement								
Volume (veh/h)	17	158	19	10	207	76		
%Thrus Left Lane								
Approach	Northbound			Southbound				
	L	T	R	L	T	R		
Movement								
Volume (veh/h)	53	42	33	53	11	20		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.77		0.77		0.77		0.77	
Flow Rate (veh/h)	251		378		164		107	
% Heavy Vehicles	6		11		0		0	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.0		0.4		0.6	
Prop. Right-Turns	0.1		0.3		0.3		0.2	
Prop. Heavy Vehicle	0.1		0.1		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.1		0.0		-0.1		-0.0	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.22		0.34		0.15		0.10	
hd, final value (s)	5.35		5.15		5.72		5.90	
x, final value	0.37		0.54		0.26		0.18	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	3.4		3.1		3.7		3.9	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	501		628		414		357	
Delay (s/veh)	11.51		14.04		10.72		10.15	
LOS	B		B		B		B	
Approach: Delay (s/veh)	11.51		14.04		10.72		10.15	
LOS	B		B		B		B	
Intersection Delay (s/veh)	12.27							
Intersection LOS	B							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Elm St. & 1st St.
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	AM Peak Hour		

Project ID

East/West Street: *Elm St.* North/South Street: *1st St.*

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	19	174	24	11	228	83
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	68	49	37	58	13	22
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.77		0.77		0.77		0.77	
Flow Rate (veh/h)	280		417		199		119	
% Heavy Vehicles	6		11		0		0	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.0		0.4		0.6	
Prop. Right-Turns	0.1		0.3		0.2		0.2	
Prop. Heavy Vehicle	0.1		0.1		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.1		0.0		-0.1		-0.0	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.25		0.37		0.18		0.11	
hd, final value (s)	5.68		5.45		6.06		6.31	
x, final value	0.44		0.63		0.34		0.21	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	3.7		3.4		4.1		4.3	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	530		637		449		369	
Delay (s/veh)	13.09		17.30		12.09		10.96	
LOS	B		C		B		B	
Approach: Delay (s/veh)	13.09		17.30		12.09		10.96	
LOS	B		C		B		B	
Intersection Delay (s/veh)	14.37							
Intersection LOS	B							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Elm St. & 1st St.
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	AM Peak Hour		

Project ID Year 2026 with Project

East/West Street: Elm St.

North/South Street: 1st St.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	19	174	30	12	228	83
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	85	62	41	58	17	22
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.77		0.77		0.77		0.77	
Flow Rate (veh/h)	287		418		243		125	
% Heavy Vehicles	6		11		0		0	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.0		0.5		0.6	
Prop. Right-Turns	0.1		0.3		0.2		0.2	
Prop. Heavy Vehicle	0.1		0.1		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.0		-0.0	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.26		0.37		0.22		0.11	
hd, final value (s)	5.93		5.71		6.23		6.57	
x, final value	0.47		0.66		0.42		0.23	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	3.9		3.7		4.2		4.6	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	537		607		493		375	
Delay (s/veh)	14.15		19.19		13.66		11.50	
LOS	B		C		B		B	
Approach: Delay (s/veh)	14.15		19.19		13.66		11.50	
LOS	B		C		B		B	
Intersection Delay (s/veh)	15.69							
Intersection LOS	C							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Elm St. & 1st St.
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	PM Peak Hour		

Project ID *Existing*East/West Street: *Elm St.*North/South Street: *1st St.*

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	45	149	67	6	106	60
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	42	53	6	52	61	38
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	0.88		0.88		0.88		0.88	
Flow Rate (veh/h)	296		194		113		171	
% Heavy Vehicles	1		17		0		0	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.2		0.0		0.4		0.3	
Prop. Right-Turns	0.3		0.4		0.1		0.3	
Prop. Heavy Vehicle	0.0		0.2		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	-0.1		0.1		0.1		-0.1	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.26		0.17		0.10		0.15	
hd, final value (s)	4.88		5.20		5.51		5.28	
x, final value	0.40		0.28		0.17		0.25	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	2.9		3.2		3.5		3.3	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	546		444		363		421	
Delay (s/veh)	11.12		10.21		9.66		10.03	
LOS	B		B		A		B	
Approach: Delay (s/veh)	11.12		10.21		9.66		10.03	
LOS	B		B		A		B	
Intersection Delay (s/veh)	10.44							
Intersection LOS	B							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Elm St. & 1st St.
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	PM Peak Hour		

Project ID Year 2026 w/o Project

East/West Street: Elm St.

North/South Street: 1st St.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	50	164	84	8	117	66
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	51	59	8	57	70	42
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.88		0.88		0.88		0.88	
Flow Rate (veh/h)	337		216		133		190	
% Heavy Vehicles	1		17		0		0	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.2		0.0		0.4		0.3	
Prop. Right-Turns	0.3		0.3		0.1		0.2	
Prop. Heavy Vehicle	0.0		0.2		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	-0.1		0.1		0.0		-0.1	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.30		0.19		0.12		0.17	
hd, final value (s)	5.09		5.46		5.79		5.56	
x, final value	0.48		0.33		0.21		0.29	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	3.1		3.5		3.8		3.6	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	587		466		383		440	
Delay (s/veh)	12.62		11.10		10.36		10.85	
LOS	B		B		B		B	
Approach: Delay (s/veh)	12.62		11.10		10.36		10.85	
LOS	B		B		B		B	
Intersection Delay (s/veh)	11.52							
Intersection LOS	B							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Elm St. & 1st St.
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	PM Peak Hour		

Project ID Year 2026 with Project

East/West Street: Elm St.

North/South Street: 1st St.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	50	164	102	12	117	66
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	62	67	11	57	83	42
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.88		0.88		0.88		0.88	
Flow Rate (veh/h)	357		220		158		205	
% Heavy Vehicles	1		17		0		0	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.2		0.1		0.4		0.3	
Prop. Right-Turns	0.3		0.3		0.1		0.2	
Prop. Heavy Vehicle	0.0		0.2		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	-0.1		0.1		0.0		-0.1	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.32		0.20		0.14		0.18	
hd, final value (s)	5.24		5.69		5.96		5.75	
x, final value	0.52		0.35		0.26		0.33	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	3.2		3.7		4.0		3.7	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	607		470		408		455	
Delay (s/veh)	13.79		11.70		11.06		11.52	
LOS	B		B		B		B	
Approach: Delay (s/veh)	13.79		11.70		11.06		11.52	
LOS	B		B		B		B	
Intersection Delay (s/veh)	12.35							
Intersection LOS	B							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Pacific Hwy. & Robb Road
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	AM Peak Hour		

Project ID Existing

East/West Street: Robb Road

North/South Street: Pacific Hwy.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	43	0	25	0	0	0
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	163	10	0	0	2	109
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.93		0.93		0.93		0.93	
Flow Rate (veh/h)	72		0		185		119	
% Heavy Vehicles	15		0		11		11	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.6		0.0		0.9		0.0	
Prop. Right-Turns	0.4		0.0		0.0		1.0	
Prop. Heavy Vehicle	0.1		0.0		0.1		0.1	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.2		0.0		0.4		-0.4	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.06		0.00		0.16		0.11	
hd, final value (s)	4.75		4.68		4.57		3.88	
x, final value	0.10		0.00		0.23		0.13	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	2.8		2.7		2.6		1.9	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	322		0		435		369	
Delay (s/veh)	8.25		7.68		8.97		7.45	
LOS	A		A		A		A	
Approach: Delay (s/veh)	8.25		7.68		8.97		7.45	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.35							
Intersection LOS	A							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Pacific Hwy. & Robb Road
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	AM Peak Hour		

Project ID Year 2026 w/o Project

East/West Street: Robb Road

North/South Street: Pacific Hwy.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	50	0	28	0	0	0
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	179	11	0	0	2	133
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.93		0.93		0.93		0.93	
Flow Rate (veh/h)	83		0		203		145	
% Heavy Vehicles	15		0		11		11	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.6		0.0		0.9		0.0	
Prop. Right-Turns	0.4		0.0		0.0		1.0	
Prop. Heavy Vehicle	0.1		0.0		0.1		0.1	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.2		0.0		0.4		-0.4	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.07		0.00		0.18		0.13	
hd, final value (s)	4.85		4.80		4.63		3.94	
x, final value	0.11		0.00		0.26		0.16	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	2.9		2.8		2.6		1.9	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	333		0		453		395	
Delay (s/veh)	8.46		7.80		9.26		7.68	
LOS	A		A		A		A	
Approach: Delay (s/veh)	8.46		7.80		9.26		7.68	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.58							
Intersection LOS	A							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Pacific Hwy. & Robb Road
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	AM Peak Hour		

Project ID Year 2026 with Project

East/West Street: Robb Road

North/South Street: Pacific Hwy.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	67	0	28	0	0	0
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	179	11	0	0	2	183
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.93		0.93		0.93		0.93	
Flow Rate (veh/h)	102		0		203		198	
% Heavy Vehicles	15		0		11		11	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.7		0.0		0.9		0.0	
Prop. Right-Turns	0.3		0.0		0.0		1.0	
Prop. Heavy Vehicle	0.1		0.0		0.1		0.1	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.2		0.0		0.4		-0.4	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.09		0.00		0.18		0.18	
hd, final value (s)	5.03		4.96		4.75		4.00	
x, final value	0.14		0.00		0.27		0.22	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	3.0		3.0		2.7		2.0	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	352		0		453		448	
Delay (s/veh)	8.86		7.96		9.48		8.13	
LOS	A		A		A		A	
Approach: Delay (s/veh)	8.86		7.96		9.48		8.13	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.82							
Intersection LOS	A							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Pacific Hwy. & Robb Road
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	PM Peak Hour		

Project ID *Existing*East/West Street: *Robb Road*North/South Street: *Pacific Hwy.*

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound				
	L	T	R	L	T	R		
Movement								
Volume (veh/h)	134	0	112	0	0	0		
%Thrus Left Lane								
Approach	Northbound			Southbound				
	L	T	R	L	T	R		
Movement								
Volume (veh/h)	68	12	0	0	7	74		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	0.83		0.83		0.83		0.83	
Flow Rate (veh/h)	295		0		95		97	
% Heavy Vehicles	6		0		3		4	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.5		0.0		0.9		0.0	
Prop. Right-Turns	0.5		0.0		0.0		0.9	
Prop. Heavy Vehicle	0.1		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	-0.1		0.0		0.2		-0.5	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.26		0.00		0.08		0.09	
hd, final value (s)	4.30		4.70		4.92		4.22	
x, final value	0.35		0.00		0.13		0.11	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	2.3		2.7		2.9		2.2	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	545		0		345		347	
Delay (s/veh)	9.62		7.70		8.65		7.77	
LOS	A		A		A		A	
Approach: Delay (s/veh)	9.62		7.70		8.65		7.77	
LOS	A		A		A		A	
Intersection Delay (s/veh)	9.06							
Intersection LOS	A							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Pacific Hwy. & Robb Road
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	PM Peak Hour		

Project ID Year 2026 w/o Project

East/West Street: Robb Road

North/South Street: Pacific Hwy.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	160	0	123	0	0	0
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	75	13	0	0	8	89
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.83		0.83		0.83		0.83	
Flow Rate (veh/h)	340		0		105		116	
% Heavy Vehicles	6		0		3		4	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.6		0.0		0.9		0.0	
Prop. Right-Turns	0.4		0.0		0.0		0.9	
Prop. Heavy Vehicle	0.1		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	-0.0		0.0		0.2		-0.5	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.30		0.00		0.09		0.10	
hd, final value (s)	4.40		4.85		5.07		4.36	
x, final value	0.42		0.00		0.15		0.14	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	2.4		2.8		3.1		2.4	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	590		0		355		366	
Delay (s/veh)	10.49		7.85		8.95		8.08	
LOS	B		A		A		A	
Approach: Delay (s/veh)	10.49		7.85		8.95		8.08	
LOS	B		A		A		A	
Intersection Delay (s/veh)	9.70							
Intersection LOS	A							

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	DSK	Intersection	Pacific Hwy. & Robb Road
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	PM Peak Hour		

Project ID Year 2026 with Project

East/West Street: Robb Road

North/South Street: Pacific Hwy.

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	209	0	123	0	0	0
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	75	13	0	0	8	121
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.83		0.83		0.83		0.83	
Flow Rate (veh/h)	399		0		105		154	
% Heavy Vehicles	6		0		3		4	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.6		0.0		0.9		0.0	
Prop. Right-Turns	0.4		0.0		0.0		0.9	
Prop. Heavy Vehicle	0.1		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		0.2		-0.5	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.35		0.00		0.09		0.14	
hd, final value (s)	4.56		5.07		5.31		4.54	
x, final value	0.51		0.00		0.15		0.19	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	2.6		3.1		3.3		2.5	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	649		0		355		404	
Delay (s/veh)	12.13		8.07		9.28		8.63	
LOS	B		A		A		A	
Approach: Delay (s/veh)	12.13		8.07		9.28		8.63	
LOS	B		A		A		A	
Intersection Delay (s/veh)	10.86							
Intersection LOS	B							

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	DSK	Intersection	I-5 NB ramps
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	AM Peak Hour		
Project Description Existing			
East/West Street: Robb Road		North/South Street: I-5 NB ramps	
Intersection Orientation: East-West		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	6	39			178	94
Peak-Hour Factor, PHF	0.81	0.81	1.00	1.00	0.81	0.81
Hourly Flow Rate, HFR (veh/h)	7	48	0	0	219	116
Percent Heavy Vehicles	5	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	17	1	74			
Peak-Hour Factor, PHF	0.81	0.81	0.81	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	20	1	91	0	0	0
Percent Heavy Vehicles	18	0	17	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		LTR				

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT			LTR				
v (veh/h)	7			112				
C (m) (veh/h)	1208			883				
v/c	0.01			0.13				
95% queue length	0.02			0.43				
Control Delay (s/veh)	8.0			9.7				
LOS	A			A				
Approach Delay (s/veh)	--	--		9.7				
Approach LOS	--	--		A				

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	DSK			Intersection	I-5 NB ramps			
Agency/Co.	Kelly Engineering			Jurisdiction	City of Kalama			
Date Performed	6/11/2021			Analysis Year	2026			
Analysis Time Period	AM Peak Hour							
Project Description Year 2026 w/o Project								
East/West Street: Robb Road				North/South Street: I-5 NB ramps				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	7	43			210	103		
Peak-Hour Factor, PHF	0.81	0.81	1.00	1.00	0.81	0.81		
Hourly Flow Rate, HFR (veh/h)	8	53	0	0	259	127		
Percent Heavy Vehicles	5	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT			TR				
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	19	1	84					
Peak-Hour Factor, PHF	0.81	0.81	0.81	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	23	1	103	0	0	0		
Percent Heavy Vehicles	18	0	17	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	0	0		
Configuration		LTR						
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT		LTR					
v (veh/h)	8		127					
C (m) (veh/h)	1156		861					
v/c	0.01		0.15					
95% queue length	0.02		0.52					
Control Delay (s/veh)	8.1		9.9					
LOS	A		A					
Approach Delay (s/veh)	--	--	9.9					
Approach LOS	--	--	A					

TWO-WAY STOP CONTROL SUMMARY								
General Information			Site Information					
Analyst	DSK		Intersection	I-5 NB ramps				
Agency/Co.	Kelly Engineering		Jurisdiction	City of Kalama				
Date Performed	6/11/2021		Analysis Year	2026				
Analysis Time Period	AM Peak Hour							
Project Description Year 2026 with Project								
East/West Street: Robb Road			North/South Street: I-5 NB ramps					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	7	46			252	111		
Peak-Hour Factor, PHF	0.81	0.81	1.00	1.00	0.81	0.81		
Hourly Flow Rate, HFR (veh/h)	8	56	0	0	311	137		
Percent Heavy Vehicles	5	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT					TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	19	1	98					
Peak-Hour Factor, PHF	0.81	0.81	0.81	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	23	1	120	0	0	0		
Percent Heavy Vehicles	18	0	17	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	0	0		
Configuration		LTR						
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT			LTR				
v (veh/h)	8			144				
C (m) (veh/h)	1097			852				
v/c	0.01			0.17				
95% queue length	0.02			0.61				
Control Delay (s/veh)	8.3			10.1				
LOS	A			B				
Approach Delay (s/veh)	--	--	10.1					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	DSK	Intersection	I-5 NB ramps					
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama					
Date Performed	6/11/2021	Analysis Year	2021					
Analysis Time Period	PM Peak Hour							
Project Description Existing								
East/West Street: Robb Road			North/South Street: I-5 NB ramps					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	5	97			103	91		
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91		
Hourly Flow Rate, HFR (veh/h)	5	106	0	0	113	99		
Percent Heavy Vehicles	2	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT					TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	25	1	221					
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91		
Hourly Flow Rate, HFR (veh/h)	27	1	242	0	0	0		
Percent Heavy Vehicles	0	0	2	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	0	0		
Configuration		LTR						
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT			LTR				
v (veh/h)	5			270				
C (m) (veh/h)	1358			916				
v/c	0.00			0.29				
95% queue length	0.01			1.23				
Control Delay (s/veh)	7.7			10.6				
LOS	A			B				
Approach Delay (s/veh)	--	--		10.6				
Approach LOS	--	--		B				

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	DSK	Intersection	I-5 NB Ramps
Agency/Co.	Kelly Engineering	Jurisdiction	city of Kalama
Date Performed	6/12/2021	Analysis Year	2026
Analysis Time Period	PM Peak Hour		
Project Description Year 2026 w/o Project			
East/West Street: Robb Road		North/South Street: I-5 NB ramps	
Intersection Orientation: East-West		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
	1	2	3	4	5	6
Movement	L	T	R	L	T	R
Volume (veh/h)	6	107			121	100
Peak-Hour Factor, PHF	0.91	0.91	1.00	1.00	0.91	0.91
Hourly Flow Rate, HFR (veh/h)	6	117	0	0	132	109
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume (veh/h)	28	1	256			
Peak-Hour Factor, PHF	0.91	0.91	0.91	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	30	1	281	0	0	0
Percent Heavy Vehicles	0	0	2	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		LTR				

Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
	1	4	7	8	9	10	11	12
Movement	LT			LTR				
Lane Configuration	LT			LTR				
v (veh/h)	6			312				
C (m) (veh/h)	1326			900				
v/c	0.00			0.35				
95% queue length	0.01			1.56				
Control Delay (s/veh)	7.7			11.1				
LOS	A			B				
Approach Delay (s/veh)	--	--		11.1				
Approach LOS	--	--		B				

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>DSK</i>	Intersection	<i>I-5 NB ramps</i>
Agency/Co.	<i>Kelly Engineering</i>	Jurisdiction	<i>City of Kalama</i>
Date Performed	<i>6/11/2021</i>	Analysis Year	<i>2026</i>
Analysis Time Period	<i>PM Peak Hour</i>		
Project Description <i>Year 2026 with Project</i>			
East/West Street: <i>Robb Road</i>		North/South Street: <i>I-5 NB ramps</i>	
Intersection Orientation: <i>East-West</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	6	115			148	106
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91
Hourly Flow Rate, HFR (veh/h)	6	126	0	0	162	116
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	28	1	297			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91
Hourly Flow Rate, HFR (veh/h)	30	1	326	0	0	0
Percent Heavy Vehicles	0	0	2	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		<i>LTR</i>				

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>			<i>LTR</i>				
v (veh/h)	6			357				
C (m) (veh/h)	1285			889				
v/c	0.00			0.40				
95% queue length	0.01			1.96				
Control Delay (s/veh)	7.8			11.7				
LOS	<i>A</i>			<i>B</i>				
Approach Delay (s/veh)	--	--		11.7				
Approach LOS	--	--		<i>B</i>				

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	DSK	Intersection	I-5 SB ramps
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	AM Peak Hour		
Project Description <i>Existing</i>			
East/West Street: <i>Robb Road</i>		North/South Street: <i>I-5 SB ramps</i>	
Intersection Orientation: <i>East-West</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		9	16	178	17	
Peak-Hour Factor, PHF	0.91	0.90	0.90	0.90	0.90	0.91
Hourly Flow Rate, HFR (veh/h)	0	10	17	197	18	0
Percent Heavy Vehicles	2	--	--	5	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			TR	LT		
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				25	3	8
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.90	0.90	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	27	3	8
Percent Heavy Vehicles	0	0	2	12	33	50
Percent Grade (%)		0			0	
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	1	0
Configuration					LTR	

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		197					38	
C (m) (veh/h)		1568					538	
v/c		0.13					0.07	
95% queue length		0.43					0.23	
Control Delay (s/veh)		7.6					12.2	
LOS		A					B	
Approach Delay (s/veh)	--	--					12.2	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	DSK	Intersection	I-5 SB ramps
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	AM Peak Hour		
Project Description Year 2026 w/o Project			
East/West Street: Robb Road		North/South Street: I-5 SB ramps	
Intersection Orientation: East-West		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		10	18	210	19	
Peak-Hour Factor, PHF	0.91	0.90	0.90	0.90	0.90	0.91
Hourly Flow Rate, HFR (veh/h)	0	11	20	233	21	0
Percent Heavy Vehicles	2	--	--	5	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			TR	LT		
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				28	3	9
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.90	0.90	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	31	3	10
Percent Heavy Vehicles	0	0	2	12	33	50
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	1	0
Configuration					LTR	

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		233					44	
C (m) (veh/h)		1562					483	
v/c		0.15					0.09	
95% queue length		0.52					0.30	
Control Delay (s/veh)		7.7					13.2	
LOS		A					B	
Approach Delay (s/veh)	--	--					13.2	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	DSK	Intersection	I-5 SB ramps
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2026
Analysis Time Period	AM Peak Hour		
Project Description Year 2026 with Project			
East/West Street: Robb Road		North/South Street: I-5 SB ramps	
Intersection Orientation: East-West		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
	1	2	3	4	5	6
Movement	L	T	R	L	T	R
Volume (veh/h)		10	18	252	19	
Peak-Hour Factor, PHF	0.91	0.90	0.90	0.90	0.90	0.91
Hourly Flow Rate, HFR (veh/h)	0	11	20	280	21	0
Percent Heavy Vehicles	2	--	--	5	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			TR	LT		
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume (veh/h)				31	3	9
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.90	0.90	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	34	3	10
Percent Heavy Vehicles	0	0	2	12	33	50
Percent Grade (%)		0			0	
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	1	0
Configuration					LTR	

Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
	1	4	7	8	9	10	11	12
Movement								
Lane Configuration		LT					LTR	
v (veh/h)		280					47	
C (m) (veh/h)		1562					415	
v/c		0.18					0.11	
95% queue length		0.65					0.38	
Control Delay (s/veh)		7.8					14.8	
LOS		A					B	
Approach Delay (s/veh)	--	--					14.8	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	DSK	Intersection	I-5 SB ramps
Agency/Co.	Kelly Engineering	Jurisdiction	City of Kalama
Date Performed	6/11/2021	Analysis Year	2021
Analysis Time Period	PM Peak Hour		
Project Description Existing			
East/West Street: Robb Road		North/South Street: I-5 SB ramps	
Intersection Orientation: East-West		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		7	26	103	29	
Peak-Hour Factor, PHF	0.91	0.80	0.80	0.80	0.80	0.91
Hourly Flow Rate, HFR (veh/h)	0	8	32	128	36	0
Percent Heavy Vehicles	2	--	--	7	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			TR	LT		
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				89	2	8
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.80	0.80	0.80
Hourly Flow Rate, HFR (veh/h)	0	0	0	111	2	9
Percent Heavy Vehicles	0	0	2	7	50	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	1	0
Configuration					LTR	

Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		128					122	
C (m) (veh/h)		1538					627	
v/c		0.08					0.19	
95% queue length		0.27					0.72	
Control Delay (s/veh)		7.6					12.1	
LOS		A					B	
Approach Delay (s/veh)	--	--					12.1	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	DSK		Intersection	I-5 SB ramps				
Agency/Co.	Kelly Engineering		Jurisdiction	City of Kalama				
Date Performed	6/11/2021		Analysis Year	2021				
Analysis Time Period	PM Peak Hour							
Project Description Year 2026 w/o Project								
East/West Street: Robb Road			North/South Street: I-5 SB ramps					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		8	29	121	32			
Peak-Hour Factor, PHF	0.91	0.80	0.80	0.80	0.80	0.91		
Hourly Flow Rate, HFR (veh/h)	0	9	36	151	39	0		
Percent Heavy Vehicles	2	--	--	7	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				98	2	9		
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.80	0.80	0.80		
Hourly Flow Rate, HFR (veh/h)	0	0	0	122	2	11		
Percent Heavy Vehicles	0	0	2	7	50	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	1	0		
Configuration					LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		151					135	
C (m) (veh/h)		1531					580	
v/c		0.10					0.23	
95% queue length		0.33					0.90	
Control Delay (s/veh)		7.6					13.1	
LOS		A					B	
Approach Delay (s/veh)	--	--					13.1	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	DSK		Intersection	I-5 SB ramps				
Agency/Co.	Kelly Engineering		Jurisdiction	City of Kalama				
Date Performed	6/11/2021		Analysis Year	2021				
Analysis Time Period	PM Peak Hour							
Project Description Year 2026 with Project								
East/West Street: Robb Road			North/South Street: I-5 SB ramps					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		8	29	148	32			
Peak-Hour Factor, PHF	0.91	0.80	0.80	0.80	0.80	0.91		
Hourly Flow Rate, HFR (veh/h)	0	9	36	184	39	0		
Percent Heavy Vehicles	2	--	--	7	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				106	2	9		
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.80	0.80	0.80		
Hourly Flow Rate, HFR (veh/h)	0	0	0	132	2	11		
Percent Heavy Vehicles	0	0	2	7	50	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	1	0		
Configuration					LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		184					145	
C (m) (veh/h)		1531					519	
v/c		0.12					0.28	
95% queue length		0.41					1.14	
Control Delay (s/veh)		7.7					14.6	
LOS		A					B	
Approach Delay (s/veh)	--	--					14.6	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	DSK			Intersection	Pacific Hwy. & site access			
Agency/Co.	Kelly Engineering			Jurisdiction	City of Kalama			
Date Performed	6/11/2021			Analysis Year	2026			
Analysis Time Period	AM Peak Hour							
Project Description Year 2026 w/o Project								
East/West Street: site access				North/South Street: Pacific Hwy.				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		66	3	4	24			
Peak-Hour Factor, PHF	1.00	0.90	0.90	0.90	0.90	1.00		
Hourly Flow Rate, HFR (veh/h)	0	73	3	4	26	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				14		14		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90		
Hourly Flow Rate, HFR (veh/h)	0	0	0	15	0	15		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		4		30				
C (m) (veh/h)		1536		940				
v/c		0.00		0.03				
95% queue length		0.01		0.10				
Control Delay (s/veh)		7.3		9.0				
LOS		A		A				
Approach Delay (s/veh)	--	--		9.0				
Approach LOS	--	--		A				

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>DSK</i>	Intersection	<i>Pacific Hwy. & site access</i>
Agency/Co.	<i>Kelly Engineering</i>	Jurisdiction	<i>City of Kalama</i>
Date Performed	<i>6/11/2021</i>	Analysis Year	<i>2026</i>
Analysis Time Period	<i>AM Peak Hour</i>		
Project Description <i>Year 2026 with Project</i>			
East/West Street: <i>site access</i>		North/South Street: <i>Pacific Hwy.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	17	66	3	4	24	11
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Hourly Flow Rate, HFR (veh/h)	18	73	3	4	26	12
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LTR</i>			<i>LTR</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	34	0	50	14	0	14
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Hourly Flow Rate, HFR (veh/h)	37	0	55	15	0	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service

Approach	Northbound	Southbound	Westbound			Eastbound			
Movement	1	4	7	8	9	10	11	12	
Lane Configuration	<i>LTR</i>	<i>LTR</i>		<i>LTR</i>			<i>LTR</i>		
v (veh/h)	18	4		30			92		
C (m) (veh/h)	1585	1536		847			927		
v/c	0.01	0.00		0.04			0.10		
95% queue length	0.03	0.01		0.11			0.33		
Control Delay (s/veh)	7.3	7.3		9.4			9.3		
LOS	A	A		A			A		
Approach Delay (s/veh)	--	--		9.4			9.3		
Approach LOS	--	--		A			A		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	DSK			Intersection	Pacific Hwy. & site access			
Agency/Co.	Kelly Engineering			Jurisdiction	City of Kalama			
Date Performed	6/11/2021			Analysis Year	2026			
Analysis Time Period	PM Peak Hour							
Project Description Year 2026 w/o Project								
East/West Street: site access				North/South Street: Pacific Hwy.				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		52	13	14	41			
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR (veh/h)	0	57	14	15	45	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				9		7		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR (veh/h)	0	0	0	10	0	7		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		15		17				
C (m) (veh/h)		1542		909				
v/c		0.01		0.02				
95% queue length		0.03		0.06				
Control Delay (s/veh)		7.4		9.0				
LOS		A		A				
Approach Delay (s/veh)	--	--		9.0				
Approach LOS	--	--		A				

TWO-WAY STOP CONTROL SUMMARY								
General Information			Site Information					
Analyst	DSK		Intersection	Pacific Hwy. & site access				
Agency/Co.	Kelly Engineering		Jurisdiction	City of Kalama				
Date Performed	6/11/2021		Analysis Year	2026				
Analysis Time Period	PM Peak Hour							
Project Description Year 2026 with Project								
East/West Street: site access			North/South Street: Pacific Hwy.					
Intersection Orientation: North-South			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	49	52	13	14	41	34		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR (veh/h)	54	57	14	15	45	37		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LTR			LTR				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	22	0	32	8	0	7		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR (veh/h)	24	0	35	8	0	7		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LTR	LTR		LTR			LTR	
v (veh/h)	54	15		15			59	
C (m) (veh/h)	1528	1542		760			828	
v/c	0.04	0.01		0.02			0.07	
95% queue length	0.11	0.03		0.06			0.23	
Control Delay (s/veh)	7.4	7.4		9.8			9.7	
LOS	A	A		A			A	
Approach Delay (s/veh)	--	--		9.8			9.7	
Approach LOS	--	--		A			A	

APPENDIX E
REFERENCES

References

1. Trip Generation Manual, 10th Edition, 2017, Institute of Transportation Engineers.
2. Highway Capacity Manual, 2000 and 2010, Transportation Research Board, National Research Council.
3. Discussions with representatives from Gray & Osborne, Inc., 8513 NE Hazel Dell Avenue, Suite 202, Vancouver, WA 98665.